STRUCTURAL NOTES

CONTRACTOR NOTE:

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. GUTHERMAN STRUCTURAL, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

CODES AND STANDARDS

1. WIND LOADS AS PER:

   A. FLORIDA BUILDING CODE 7TH EDITION (2020) WITH AN ULTIMATE DESIGN WIND SPEED OF 135 MPH, EXPOSURE C, NOMINAL DESIGN WIND SPEED OF 104 MPH, +/-0.18 INTERNAL PRESSURE COEFFICIENT, AND BUILDING RISK CATEGORY II.

   B. THIS BUILDING IS DESIGNED AS AN ENCLOSED BUILDING.

2. SEISMIC SITE CLASS = E
   SEISMIC DESIGN CATEGORY = B
   GROUND SNOW LOAD, Pg = 0 PSF
   FLOOD ZONE = X
   RAIN INTENSITY = 4.5 INCHES PER HOUR (100 YEAR)

3. THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:

   A. FLORIDA BUILDING CODE 7TH EDITION (2020).

   B. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318/ 2014 EDITION).

   C. MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315/ LATEST EDITION).

   D. NATIONAL DESIGN SPECIFICATION, WOOD CONSTRUCTION NDS/2018 EDITION. DESIGN USING ASD (ALLOWABLE STRESS DESIGN) METHOD.

4. ARCHITECTURAL AND MECHANICAL DRAWINGS:

   A. THE STRUCTURAL DRAWINGS ARE PART OF THE CONTRACT DOCUMENTS AND DO NOT BY THEMSELVES PROVIDE ALL THE INFORMATION REQUIRED TO PROPERLY COMPLETE THE PROJECT STRUCTURE. THE GENERAL CONTRACTOR SHALL CONSULT THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND COORDINATE THE INFORMATION CONTAINED IN THESE DRAWINGS WITH THE STRUCTURAL DRAWINGS TO PROPERLY CONSTRUCT THE PROJECT.

   B. REFER TO ARCHITECTURAL, MECHANICAL OR ELECTRICAL DRAWINGS FOR ADDITIONAL OPENINGS, DEPRESSIONS, FINISHES, INSERTS, BOLTS SETTINGS, DRAINS, REGLETS, ETC.

   C. BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.

   D. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.

   E. ALL STRUCTURES HAVE BEEN DESIGNED TO RESIST THE DESIGN LOADS LISTED ONLY AS COMPLETED STRUCTURES. THE GENERAL CONTRACTOR SHALL FULLY BRACE AND OTHERWISE PROTECT WORK IN PROGRESS UNTIL THE STRUCTURES ARE COMPLETED. THE GENERAL CONTRACTOR SHALL ALSO INSURE THAT ITS OPERATIONS AND PROCEDURES PROVIDE NO LOADING GREATER THAN THE DESIGN LOADS LISTED ON ANY MEMBER.

5. SECTIONS AND DETAILS:

   ALL DETAILS, SECTIONS AND NOTES SHOWN ON THE DRAWINGS ARE INTENDED TO BE TYPICAL AND SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS OTHERWISE SHOWN.

6. MATERIALS AND ASSEMBLY TEST AS FOLLOWS:

   A. EXTERIOR WINDOWS, SLIDING AND PATIO GLASS DOORS SHALL BE TESTED BY AN APPROVED INDEPENDENT TESTING LABORATORY, AND SHALL BE LABELED WITH AN APPROVED LABEL IDENTIFYING THE MANUFACTURER, PERFORMANCE CHARACTERISTICS AND APPROVED PRODUCT CERTIFICATION AGENCY, TESTING LABORATORY, EVALUATION ENTITY OR FLORIDA STATE WIDE PRODUCT APPROVAL NUMBER TO INDICATE COMPLIANCE WITH THE REQUIREMENTS OF ONE OF THE FOLLOWING SPECIFICATIONS:

   ANSI/AAMA/NWWDA 101/I.S. 2-97 OR TAS 202 (HVHZ SHALL COMPLY WITH TAS 202)

   B. EXTERIOR DOOR ASSEMBLIES SHALL BE TESTED FOR STRUCTURAL INTEGRITY IN ACCORDANCE WITH ASTM E330 AT A LOAD OF 1.5 TIMES THE REQUIRED DESIGN PRESSURE LOAD. THE LOAD SHALL BE SUSTAINED FOR 10 SECONDS WITH NO PERMANENT DEFORMATION OF ANY MAIN FRAME OR PANEL MEMBER IN EXCESS OF 0.4 PERCENT OF ITS SPAN AFTER THE LOAD IS REMOVED. HVHZ SHALL COMPLY WITH TAS 202. AFTER EACH SPECIFIED LOADING, THERE SHALL BE NO GLASS BREAKAGE, PERMANENT DAMAGE TO FASTENERS, HARDWARE PARTS, OR ANY OTHER DAMAGE, WHICH CAUSES THE DOOR TO BE INOPERABLE.

   C. WINDOW AND DOOR ASSEMBLIES SHALL BE ANCHORED IN ACCORDANCE WITH THE PUBLISHED MANUFACTURER'S RECOMMENDATIONS TO ACHIEVE THE DESIGN PRESSURE SPECIFIED. SUBSTITUTE ANCHORING SYSTEM USED FOR SUBSTRATES NOT SPECIFIED BY THE FENSTRATION MANUFACTURER SHALL PROVIDE EQUAL OR GREATER ANCHORING PERFORMANCE AS DEMONSTRATED BY ACCEPTED ENGINEERING PRACTICE.

SPECIALTY ENGINEERED PRODUCTS

1. THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY. SPECIALTY ENGINEERED PRODUCTS SHALL BE, BUT ARE NOT LIMITED TO, THE FOLLOWING:

   A. LIGHT GAUGE METAL, INCLUDING BUT NOT LIMITED TO, SOFFITS, CLADDING, CEILINGS, ETC.

   B. MISCELLANEOUS METALS INCLUDING STEEL STAIRS, MECHANICAL EQUIPMENT SUPPORTS, FRAMES THAT SUPPORT MACHINES, PIPES OR OTHER STRUCTURAL METAL USED FOR SUPPORT OF MECHANICAL SYSTEMS.

   C. MISCELLANEOUS HANGERS, CHANDELIERS, CABINETS, METAL FRAMES, LADDERS, RIGGING, HANGING WALLS, RAILINGS, GLAZING FRAMES, CLADDING SUCH AS STONE, PRECAST, ALUMINUM, METAL PANELS, CABLE BARRIER SYSTEMS, ETC. OR ANY OTHER MISCELLANEOUS PRODUCT REQUIRED BY ANY OF THE CONSTRUCTION DOCUMENTS.

   D. IN ADDITION TO THE LOADS SHOWN IN THE DESIGN LOAD SCHEDULE, THE SPECIALTY ENGINEER SHALL DESIGN FOR THE WEIGHT OF ALL MECHANICAL, PLUMBING AND ELECTRICAL EQUIPMENT AND FIXTURES, AS WELL AS CHANDELIER FIXTURES, BAR CABINETS, AND ART WORK / MOBILES.

GENERAL CONTRACTOR TO INCLUDE IN THEIR BID THE COST OF THE ABOVE NOTED SPECIALTY ENGINEERING.

FOUNDATION

1. ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE REPORT ON SOILS AND FOUNDATION INVESTIGATION RECOMMENDATIONS ON SOILS AND FOUNDATIONS INVESTIGATION PREPARED BY AN APPROVED TESTING LABORATORY PRIOR TO FOUNDATION WORK.

2. BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2000 PSF.

3. SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.

4. ALL MONOLITHIC EDGE FOOTINGS SHALL BEAR A MINIMUM 1'-0" BELOW EXTERIOR GRADE TYP. MAKE ADJUSTMENTS AS NEEDED.

5. EXCAVATION & BACKFILL:

   A. ALL EXCAVATION SHALL BE KEPT DRY. EXCAVATE TO DEPTHS AND DIMENSIONS INDICATED. TAKE EVERY PRECAUTION TO GUARD AGAINST ANY MOVEMENT OR SETTLEMENT OF ADJACENT STRUCTURES, UTILITIES, PIPING, ETC.

   B. PROVIDE ANY BRACING OR SHORING NECESSARY TO AVOID SETTLEMENT OR DISPLACEMENT OF EXISTING FOUNDATION OR STRUCTURES.

6. DIMENSIONS: ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

CONCRETE

1. ALL CONCRETE SHALL BE 3000 PSI READY MIX AND MEET THE FOLLOWING REQUIREMENTS:

   A. SLUMPS SHALL BE 4-INCHES MINIMUM AND 6-INCHES MAXIMUM.
   B. CONCRETE SHALL HAVE 3 PERCENT AIR ENTRAINMENT.
   C. ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55.
   D. JOBSITE WATER SHALL NOT BE ADDED.
   E. CEMENT SHALL CONFORM WITH ASTM C150 TYPE 1. SLAG, ASTM C989 SHALL BE LIMITED TO 50% (BY WEIGHT OF CEMENTITIOUS MATERIAL AND FLY ASH, ASTM C618, CLASS F, SHALL BE LIMITED TO 25% (BY WEIGHT) OF CEMENTITIOUS MATERIAL.

2. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE ACI BUILDING CODE (ACI 318/ 2014 EDITION), THE ACI DETAILING MANUAL (ACI 315/ 2004 EDITION), AND THE SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301/ LATEST EDITION).

3. SUBMIT ALL REINFORCING STEEL SHOP DRAWINGS FOR APPROVAL PRIOR TO ANY FABRICATION.

4. CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.

5. WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 1064, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.

6. LAP ALL BARS WITH CLASS B TENSION LAP SPLICE UNLESS OTHERWISE NOTED ON DRAWINGS. LAP ALL WWF A MINIMUM OF 12 INCHES (UNLESS OTHERWISE NOTED).

7. REINFORCING BARS:

   A. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.

   B. AT CORNERS OF CONCRETE WALLS, BEAMS AND CONTINUOUS WALL FOOTINGS, PROVIDE (1-#5 OR MATCHING) HORIZONTAL BARS X 5'-0" BENT BAR FOR EACH HORIZONTAL BAR SCHEDULED AT EACH FACE.

   C. ALL HOOKS SHOWN IN REINFORCEMENT SHALL BE ACI RECOMMENDED HOOKS UNLESS OTHERWISE NOTED.

WOOD

1. ALL STRUCTURAL WOOD MEMBERS ARE DESIGNED AS "DRY-USE". MOISTURE CONTENT MUST BE 19% OR LESS. STORE WOOD FRAMING ABOVE GROUND AND UNDER TARPS WITH PROPER AIR CIRCULATION.

2. ALL LUMBER SHALL BE SOUTHERN PINE SPECIES #2 GRADE OR APPROVED EQUAL. ALLOWABLE DESIGN STRESSES SHALL FOLLOW NATIONAL DESIGN SPECIFICATION (NDS) (LATEST EDITION).

3. HEADERS AT NON BEARING CONDITIONS SHALL BE AS FOLLOWS:

OPENING SIZE	HEADER
UP TO 4' -0"	(2) 2" X 6"
4'- 0" TO 6'- 0"	(2) 2" X 8"
6'- 0" TO 9'- 0"	(2) 2" X 10"

4. PROVIDE SP ACQ PRESSURE TREATED LUMBER IN ACCORDANCE WITH AWPA STANDARDS TO A MINIMUM 0.40 PCF RETENTION WHERE LUMBER IS IN CONTACT WITH CONCRETE/MASONRY OR OUTSIDE OF BUILDING. ALL METAL CONNECTORS IN CONTACT WITH PRESSURE TREADED LUMBER SHALL BE GALVANIZED WITH A RATING OF G-185 AND CONFORM TO ASTM A653. ALL NAILS AND SCREWS USED WITH PRESSURE TREATED LUMBER ARE TO BE HOT-DIPPED GALVANIZED AND TO CONFORM TO ASTM A153 CLASS D. ELECTROGALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM NOT TO BE USED IN DIRECT CONTACT WITH ACQ TREATED LUMBER.

5. PLYWOOD SHEATHING:
   A. FLOOR: USE 3/4" T&G APA 24oc STURD-I-FLOOR, EXP. 1, PLYWOOD SUB-FLOOR SHEATHING, HUBER BLUE PLUS OSB, OR EQUAL.

   B. WALL: Use 19/32" APA 32/16 MIN. RATED, EXP. 1, PLYWOOD SHEATHING. FIRST 48" OF WALL FROM GRADE SHALL HAVE PRESSURE TREATED PLYWOOD, OR ZIP SHEATHING. ATTACH TO STUDS WITH 8d NAILS AT 6" O.C. IN FIELD OF PANEL, AND 12" O.C. ALL OTHER SUPPORTS. FOR SHEAR WALLS, REFER TO THE SHEAR WALL SCHEDULE FOR FASTENER REQUIREMENTS.

   C. ROOF: Use 19/32"-40/20 RATED, EXP. 1, PLYWOOD SHEATHING. ATTACH TO TRUSSES WITH 10d RING SHANK NAILS AT 6" O.C. IN FIELD OF PANEL, AND 12" O.C. ALL OTHER SUPPORTS.

   D. SEE FRAMING PLANS FOR NAILING AND/OR BLOCKING REQUIREMENTS. USE 8'- 0" LONG X 4'-0" WIDE SHEETS WITH LENGTH ACROSS FRAMING. STAGGER PANEL END JOINTS 4'-0" TYP., ALLOW 1/8" SPACE ALONG PANEL EDGES AND END JOINTS.

   E. FLOOR SHEATHING TO BE NAILED WITH 10d NAILS AT 6" O.C. AND GLUED FOR PARTIAL COMPOSITE ACTION. SELECT ADHESIVE WITH APA AFG-01 SPECIFICATION AND FOLLOW APA RECOMMENDATIONS.

6. WOOD CONNECTIONS - ALL NAILS USED FOR STRUCTURAL FRAMING MEMBERS SHALL BE COMMON WIRE, U.N.O. ALL NAILS, TRUSS HANGERS, TRUSS ANCHORS AND STRAPS SHALL BE GALVANIZED FOR CORROSIVE RESISTANCE. ALL METAL STRAPS MUST BE INSTALLED WITH EQUAL LENGTHS ABOUT THE JOINT LINE. USE SIMPSON STRONG-TIE CONNECTOR PRODUCTS OR APPROVED EQUAL. TOE NAILING WILL NOT BE PERMITTED.

7. ALL NON-SHEAR WALL SILL PLATES FOR EXTERIOR BEARING WALLS SHALL BE ATTACHED TO THE FOUNDATIONS WITH 1/2" DIMATER J-BOLTS (5" EMBED) AT 48" O.C. TYP.

   INTERIOR WALLS CAN BE ATTACHE WITH 1/2" DIAMETER TAPCONS (4" MIN. EMBED) OR 1/2" DIMATER J-BOLTS (5" EMBED) AT 48" O.C. TYP. PROVIDE 3" SQUARE WASHERS ON BUILDINGS WITHOUT CONTINUOUS TIE ROD SYSTEMS ONLY.

TIMBER

1. ALL MICROLLAM LVL BEAMS TO

   A. BE ENGINEERED AND MANUFACTURED BY TRUS JOIST WEYERHAEUSER (TJW) OR APPROVED EQUAL. TEMPORARY BRACING TO BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS. PROVIDE CONTINUOUS SUPPORT OF THE COMPRESSION EDGE AND PROVIDE LATERAL SUPPORT AT ALL BEARINGS. THE MINIMUM ALLOWABLE STRESSES FOR MICROLLAM BEAMS ARE AS FOLLOWS:

   Fb = 2,600 PSI Fv = 285 PSI E = 1,900,000 PSI

   B. CONNECT: (2) PLY LVL W/ (3) ROWS OF 16d COMMON NAILS AT 12" O/C;

   C. CONNECT: (3) PLY LVL W/ (3) ROWS STAGGERED OF 1/2" DIAMETER A307 GRADE OR BETTER THRU-BOLTS SPACED AT 24" O/C, STARTING THE TOP AND BOTTOM ROWS 2" FROM BEAM ENDS TYP.

2. ALL STRUCTURAL TIMBER TO

   A. BE DOUGLAS FIR SPECIES, #2 GRADE (MINIMUM) OR APPROVED EQUAL.

   B. SOUTHERN PINE SPECIES, #2 GRADE (MINIMUM) OR APPROVED EQUAL.

   C. SOUTHERN YELLOW PINE STRUCTURAL SELECT Fb=2100 E=1,800,000

   D. BE DESIGNED PER THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION'S (AITC)"TIMBER CONSTRUCTION MANUAL" AND AMERICAN FOREST & PAPER ASSOCIATION'S (AFPA) NATIONAL DESIGN SPECIFICATION".

   E. PROVIDE SP ACQ PRESSURE TREATED LUMBER IN ACCORDANCE WITH AWPA STANDARDS TO A MINIMUM 0.40 PCF RETENTION WHERE LUMBER IS IN CONTACT WITH CONCRETE/MASONRY OR OUTSIDE OF BUILDING. ALL METAL CONNECTORS IN CONTACT WITH PRESSURE TREADED LUMBER SHALL BE GALVANIZED WITH A RATING OF G-185 AND CONFORM TO ASTM A653. ALL NAILS AND SCREWS USED WITH PRESSURE TREATED LUMBER ARE TO BE HOT-DIPPED GALVANIZED AND TO CONFORM TO ASTM A153 CLASS D. ELECTROGALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM NOT TO BE USED IN DIRECT CONTACT WITH ACQ TREATED LUMBER.

WOOD TRUSSES

1. WOOD ROOF TRUSSES, AND FLOOR TRUSSES ARE TO BE DESIGNED FOR THE WOOD FABRICATOR BY A PROFESSIONAL SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA. SEALED CALCULATIONS AND LAYOUT DRAWINGS ARE TO BE SUBMITTED FOR APPROVAL. TRUSS FABRICATOR TO PROVIDE ALL TRUSS-TO-TRUSS HANGERS AS REQUIRED TO RESIST GRAVITY AND UPLIFT REACTION. (UPLIFT LOADING SHALL USE COMPONENTS & CLADDING WIND FORCES.)

2. WOOD TRUSSES SHALL BE BRACED AND ERECTED IN ACCORDANCE WITH THE 2008 EDITION OF THE BUILDING COMPONENT SAFETY INFORMATION GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES, JOINTLY PRODUCED BY WTCA AND TRUSS PLATE INSTITUTE. BRACING IN THE PLANE OF THE WEB MEMBERS:

   A. THE TRUSS FABRICATOR SHALL PROVIDE AND LOCATE CONTINUOUS LATERAL BRACING FOR EACH TRUSS WEB MEMBER AS REQUIRED.

   B. LATERAL BRACING SHALL BE RESTRAINED BY DIAGONAL BRACING (MIN. 2" THICK NOMINAL LUMBER). THIS BRACING IS TO BE CONTINUOUS.

   C. A MINIMUM OF TWO ROWS OF DIAGONAL BRACING IS REQUIRED, ONE AT EACH VERTICAL WEB MEMBER CLOSEST TO BEARING LOCATIONS.

3. THE BOTTOM CHORDS SHALL BE BRACED BY CONTINUOUS LATERAL BRACING SPACED AT 8'-0" ON CENTER WITH A CEILING ATTACHED TO BOTTOM OF TRUSSES. IF NO CEILING IS ATTACHED TO BOTTOM OF TRUSSES, BRACING SHALL BE MINIMUM 2X4 @ 36" ON CENTER NAILED TO THE TOP OF THE BOTTOM CHORD. DIAGONALS PLACED AT 45 DEGREES TO THE LATERAL BRACES SHALL BE LOCATED AT EACH END. IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20 FOOT INTERVALS.

4. TOP CHORD BRACING:

   A. IF PLYWOOD DECKING IS APPLIED DIRECTLY TO TOP CHORD, PROPERLY LAPPED AND NAILED TO DEVELOP DIAPHRAGM ACTION, BRACING IS NOT REQUIRED.

   B. IF PURLINS ARE USED, DIAGONAL TOP CHORD BRACING IS REQUIRED AT EACH END. IF BUILDING EXCEEDS 60 FEET IN LENGTH, DIAGONAL BRACING SHOULD BE REPEATED AT 20-FOOT INTERVALS.

5. DO NOT CUT, DRILL OR NOTCH ROOF OR FLOOR TRUSSES WITHOUT WRITTEN APPROVAL FROM TRUSS ENGINEER. COORDINATE MECHANICAL, ELECTRICAL, PLUMBING, ETC. SIZES AND LOCATIONS WITH TRUSS LAYOUT PRIOR TO ERECTION.

6. TRUSSES SHALL BE MANUFACTURED & DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATION(S) FOR WOOD CONSTRUCTION, AF & PA, AND NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION, ANSI/TPI 1-2007, AND THE LOCAL CODE JURISDICTIONS.

7. DO NOT OVERLOAD FLOOR OR ROOF TRUSSES WITH BUILDING MATERIALS.

8. CONNECTOR PLATES SHALL BE MANUFACTURED BY A WTCA MEMBER PLATE SUPPLIER AND SHALL MEET OR EXCEED ASTM A653/A653M REQUIREMENTS FOR STRUCTURAL STEEL. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER, AND INCLUDED IN THE DESIGN PACKAGE.

SHOP DRAWINGS

1. THE SHOP DRAWINGS SHALL BE SUBMITTED IN COMPLETE PACKAGES FOR THE FOLLOWING:

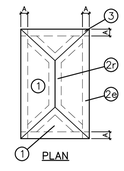
   A. CONCRETE MIX DESIGNS
   B. CONCRETE REINFORCING STEEL AND WELDED WIRE FABRIC
   C. LUMBER FRAMING PRODUCTS
   D. PRE-ENGINEERED WOOD TRUSSES
   E. PRE-ENGINEERED CONTINUOUS ROD HOLD DOWN SYSTEM FOR SHEAR WALLS AND UNIFORM ROOF UPLIFT RODS

2. PRE-ENGINEERED ITEMS SHALL BE SUBMITTED SIGNED AND SEALED BY A SPECIALTY ENGINEER REGISTERED IN THE STATE OF FLORIDA.

ROOF WIND PRESSURE (PSF)

ROOF AREA			
1	2e	2r	3
+17.2/-33.4	+17.2/-41.1	+17.2/-52.5	+17.2/-41.1

1. +: INDICATES WIND PRESSURE
   -: INDICATES WIND SUCTION
2. WALL DISTANCE A = 4.8 FT (COMPONENTS AND CLADDING)
3. FOR EFFECTIVE WIND AREAS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE WIND AREA

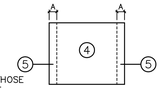


DOOR & WINDOW WIND PRESSURE (PSF) Vasd
COMPONENTS AND CLADDING-EXPOSURE B

SIZE OF WALL OPENING (SQ. FT.)	WALL AREA	
	4	5
10	+26.7/-28.9	+26.7/-35.7
20	+25.5/-27.7	+25.5/-33.3
50	+23.9/-26.1	+23.9/-30.1
100	+22.7/-24.9	+22.7/-27.7

NOTES:

1. +: INDICATES WIND PRESSURE
   -: INDICATES WIND SUCTION
2. WALL DISTANCE A = 4.8 FT (COMPONENTS AND CLADDING)
3. FOR WALL OPENINGS BETWEEN THOSE GIVEN ABOVE THE LOAD MAY BE INTERPOLATED, OTHERWISE USE THE LOAD ASSOCIATED WITH THE LOWER WALL OPENING AREA.



WALL ELEVATION

CONCRETE COVER SCHEDULE

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 OR LARGER	2"
#5 OR SMALLER	1 1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS (#11 AND SMALLER)	3/4"
BEAMS, COLUMNS (PRIMARY REINF., TIES, STIRRUPS, SPIRALS)	1 1/2"
CONCRETE FOR COASTAL EXPOSURES EXPOSED TO WEATHER:	
WALL & SLABS	2"
OTHER MEMBERS	2 1/2"

HEADER SCHEDULE

MARK	SIZE (IN.)	BEARING STUDS
H-1	(2) 2 X 6	(1.5") B & (2) FULL HEIGHT
H-2	(2) 2 X 10	(3") B & (2) FULL HEIGHT
H-3	(3) 2 X 6	(1.5") B & (2) FULL HEIGHT
H-4	(3) 2 X 8	(3") B & (2) FULL HEIGHT
H-5	(3) 2 X 10	(3") B & (2) FULL HEIGHT
H-6	(2)1.5 X 7.25 MICROLLAM LVL 2.0E-2600Fb	(4.5") B & (2) FULL HEIGHT
H-7	(2)1.75 X 7.25 PARALLAM PSL 2.2E-2900Fb	(4.5") B & (2) FULL HEIGHT

NOTE:
1. B INDICATES BEARING STUDS, TOTAL WIDTH OF SUPPORT.
2. PROVADE 1/2" INSULATION BOARDS OR PLYWOOD BETWEEN EXTERIOR HEADERS TO FLUSH WITH WALL WIDTH.

"SIMPSON" TRUSS TIE DOWN (U.N.O.)

MARK	ANCHOR TYPE	NAILS TO TRUSS	NAILS TO PLATE	NAILS TO STUD	BOLTS	ALLOWABLE UPLIFT	LATERAL LOAD PARALLEL TO WALL	LATERAL LOAD PERPEND. TO WALL
(A)	H2.5A	5-8d	5-8d	5-8d	-	600 #	110 #	110 #
(B)	H10A**	9-10dx1 1/2"	9-10dx1 1/2"	9-10dx1 1/2"	-	1340 #	590 #	285 #
(C)	H16 / 16-2	2-10dx1 1/2"	10-10dx1 1/2"	9-10dx1 1/2"	-	1340 #	590 #	285 #
(D)	MGT	22-10d	-	-	5/8"ø	3965 #	-	-
(E)	LGT3-SDS2.5 *WHEN G.T. IS ABOVE HEADER, PROVIDE AN ST2215 AT EACH SIDE OF HEADER TO BEARING STUDS. PROVIDE (2) CS18 ACROSS FLOORS AND AN HDU2 AT FOUNDATION FOR EACH SIDE	12-SDS x 2 1/2"	26-16d	26-16d	-	9715 # MIN	-	-
(F)	H7Z	4-8d	2-8d	8-8d	-	985 #	400 #	-

**WHEN CONNECTOR (B) DOES NOT FIT DUE TO TRUSS PRESS PLATE INTERFERENCE, SUBSTITUTE (C) CONNECTOR

PLYWOOD SHEATHING NAILING SCHEDULE
ROOF AND WALL (NOT FOR SHEAR WALLS)

NAIL SIZE	NAIL SPACING	ZONE
8d (RING SHANK)	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	ROOF (2) (3)
8d (RING SHANK)	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	ROOF (1)
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	WALL (5)
8d	6" @ EDGES, 12" @ INTERMEDIATE SUPPORTS	WALL (4)

CORNER DISTANCE, A = 4.8 FEET

DESIGN LOAD SCHEDULE
(ALL LOADS SHOWN ARE IN POUNDS PER SQ. FT.)

COMPONENT \ AREA	2ND FLOOR	ROOF	SLAB ON GRADE			
TOP CHORD DEAD	15	10				
BOTTOM CHORD DEAD	10	10				
TOTAL DEAD LOAD	25	20				
TOTAL LIVE LOAD (TOP CHORD)	40	20	100			
TOTAL LOAD	65	40	100			

WALL FRAMING SCHEDULE

LOCATION	SYP #2 TYP.
GROUND FLOOR (EXT)	2x6 @ 16" O.C.
GROUND FLOOR (INT)	2x4 @ 16" O.C.
SECOND FLOOR	2x4 @ 16" O.C.

* FINGER JOINTED STUDS ARE NOT ALLOWED.

FOOTING SCHEDULE

MARK	SIZE W x D x L	REINFORCING
MF-1	18" x 20" DEEP x CONT.	(2)#5 CONT. BOTTOM
MF-2	24" x 16" DEEP x CONT.	(3)#5 CONT. BOTTOM
MF-3	18" x 16" DEEP x CONT.	(2)#5 CONT. BOTTOM
TS25	2'-6"x 20"x 2'-6"	(3)#5 EACH WAY BOTTOM

NOTE:
1. FOOTING SIZES SHOWN SHALL BE USED FOR CONVENTIONAL MONOLITHC FOUNDATIONS AND FOR POST TENSION SLAB FOUNDATIONS

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LEVEL ELEVEN STUDIO INC.
220 SANDLEWOOD TRL
WINTER PARK, FL. 32789
407.519.9157

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Townhomes
Thompkins Dr, Osceola County, FL 34771

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Matt Phelps
Fl. License No. AR98401

S0.1

MARK	LOCATION	INTERIOR TENANT WALL			AIR-GAP, NO SHEATHING			BASE CONNECTION AT B.O. SHEAR WALL			
		SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	UPLIFT ANCHOR	BOTTOM SILL PLATE ATTACHMENT	# OF STUDS AT WALL ENDS	
SW-1	GROUND TO 2ND	5/8" TYPE "X" GYPSUM SHEATHING BOARD	6d COOLER OR WALLBOARD NAIL @ 4" O.C.	6d COOLER OR WALLBOARD NAIL @ 7" O.C.	–	–	–	SIMPSON DTT2Z	1/2"ø ANCHOR BOLTS 6" EMBED AT 48" O.C.	(2)2X6	92
	2ND TO ROOF	5/8" TYPE "X" GYPSUM SHEATHING BOARD	6d COOLER OR WALLBOARD NAIL @ 4" O.C.	6d COOLER OR WALLBOARD NAIL @ 7" O.C.	–	–	–	(2) CS20	(2) 0.131ø NAILS AT 48" O.C.	(2)2X4	39
MARK	LOCATION	INTERIOR WALL (BOTH SIDES OF WALL)						BASE CONNECTION AT B.O. SHEAR WALL			
		SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	UPLIFT ANCHOR	BOTTOM SILL PLATE ATTACHMENT	# OF STUDS AT WALL ENDS	
SW-2	GROUND TO 2ND	5/8" TYPE "X" GYPSUM SHEATHING BOARD	6d COOLER OR WALLBOARD NAIL @ 4" O.C.	6d COOLER OR WALLBOARD NAIL @ 7" O.C.	–	–	–	SIMPSON HDU4	1/2"ø ANCHOR BOLTS 6" EMBED AT 24" O.C.	(2)2X6	200
MARK	LOCATION	EXTERIOR SIDE			INTERIOR UNIT SIDE			BASE CONNECTION AT B.O. SHEAR WALL			
		SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	UPLIFT ANCHOR	BOTTOM SILL PLATE ATTACHMENT	# OF STUDS AT WALL ENDS	
SW-3	GROUND TO 2ND	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	–	–	–	SIMPSON HDU4	1/2"ø ANCHOR BOLTS 6" EMBED AT 24" O.C.	(2)2X6	214
	2ND TO ROOF	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	–	–	–	(2) CS20	(2) 0.131ø NAILS AT 24" O.C.	(2)2X4	82
MARK	LOCATION	EXTERIOR SIDE			INTERIOR SIDE			BASE CONNECTION AT B.O. SHEAR WALL			
		SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	UPLIFT ANCHOR	BOTTOM SILL PLATE ATTACHMENT	# OF STUDS AT WALL ENDS	
SW-4	GROUND TO 2ND	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	SIMPSON HDU4	1/2"ø ANCHOR BOLTS 6" EMBED AT 16" O.C.	(3)2X6	331
	2ND TO ROOF	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	5/8" TYPE "X" GYPSUM SHEATHING BOARD	6d COOLER OR WALLBOARD NAIL @ 4" O.C.	6d COOLER OR WALLBOARD NAIL @ 7" O.C.	SIMPSON HDU2	(2) 0.131ø NAILS AT 16" O.C.	(3)2X4	141
MARK	LOCATION	INTERIOR UNIT SIDE (BOTH SIDES OF WALL)						BASE CONNECTION AT B.O. SHEAR WALL			
		SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	UPLIFT ANCHOR	BOTTOM SILL PLATE ATTACHMENT	# OF STUDS AT WALL ENDS	
SW-5	GROUND TO 2ND	5/8" TYPE "X" GYPSUM SHEATHING BOARD	6d COOLER OR WALLBOARD NAIL @ 4" O.C.	6d COOLER OR WALLBOARD NAIL @ 7" O.C.	–	–	–	SIMPSON HDU4	5/8"ø ANCHOR BOLTS 6" EMBED AT 24" O.C.	(2)2X6	209
	2ND TO ROOF	5/8" TYPE "X" GYPSUM SHEATHING BOARD	6d COOLER OR WALLBOARD NAIL @ 4" O.C.	6d COOLER OR WALLBOARD NAIL @ 7" O.C.	–	–	–	(2) CS20	(2) 0.162ø NAILS AT 18" O.C.	(2)2X4	73
MARK	LOCATION	EXTERIOR SIDE			INTERIOR SIDE			BASE CONNECTION AT B.O. SHEAR WALL			
		SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	SHEATHING MATERIAL	EDGE NAILING	FIELD NAILING	UPLIFT ANCHOR	BOTTOM SILL PLATE ATTACHMENT	# OF STUDS AT WALL ENDS	
SW-6	GROUND TO 2ND	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	7/16" PLYWOOD	8d AT 6" O.C.	8d AT 12" O.C.	SIMPSON HDU11	1/2"ø ANCHOR BOLTS 6" EMBED AT 16" O.C.	(2)2X6	250

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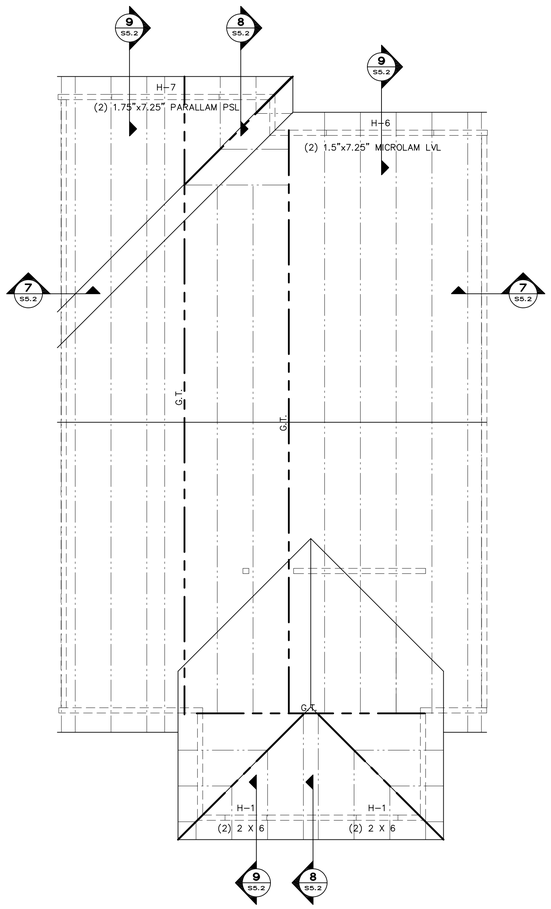
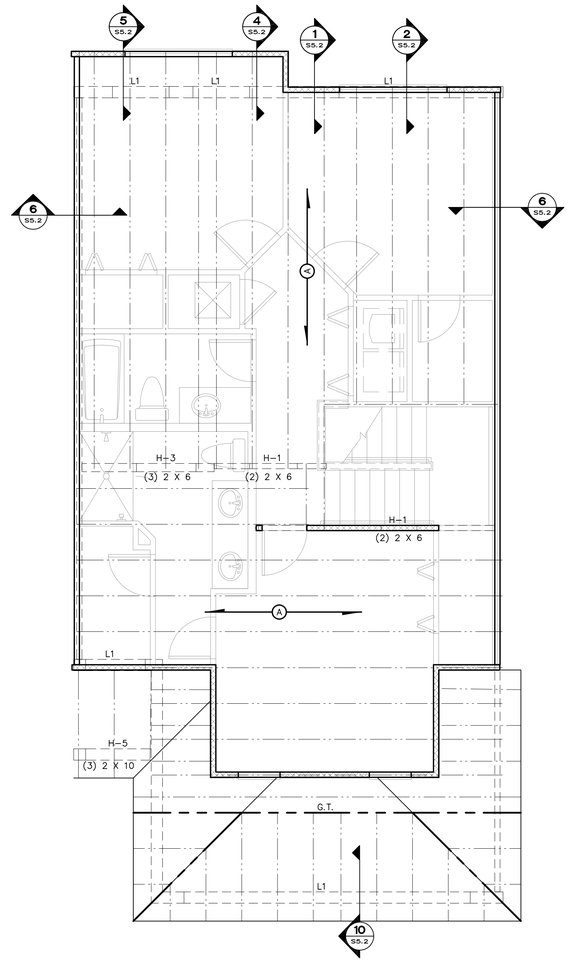
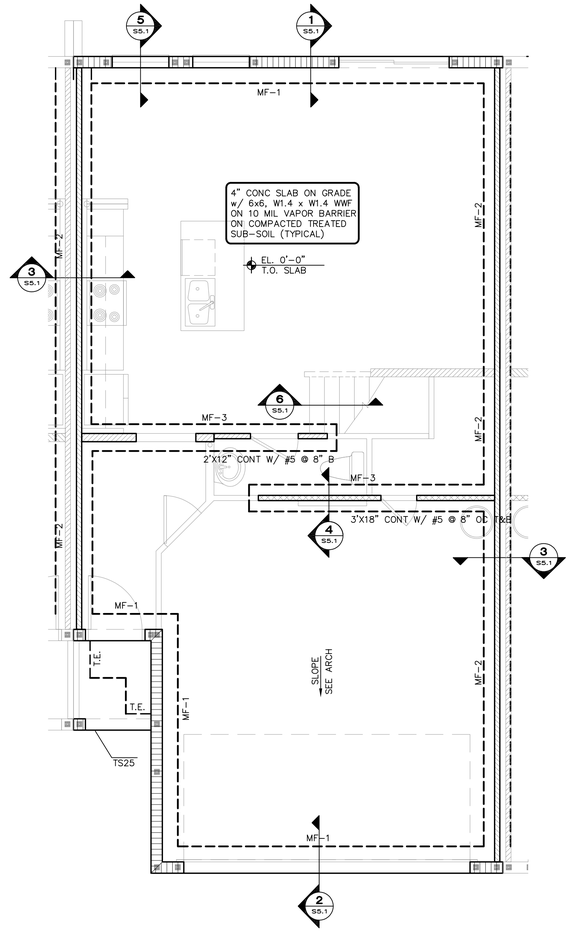
S0.2







3 UNIT A1 ROOF FRAMING PLAN

1/4" = 1'-0"

ROOF FRAMING NOTES:

1. FLOOR MEMBER BEARING ELEVATION IS +18'-11 7/8" U.N.O.
2. ROOF WOOD TRUSSES SHALL BE SPACED 2'-0" O.C. MAX.
3. G.T. INDICATES ROOF GIRDER TRUSS BY TRUSS MANUFACTURER.
4. H-# INDICATES WOOD HEADER, SEE SCHEDULE ON SHEET S0.1.
5. RUNS FOR MECH'L, ELECTRICAL AND PLUMBING (MEP) THROUGH PRE-FABRICATED TRUSSES MUST BE COORDINATED WITH THE TRUSS DESIGNER AND MEP DWGS. (PIPING, DUCT RUNS, ETC.) FIRE SPRINKLER RUNS MUST ALSO BE COORDINATED.
6. ALL ROOF TRUSS UPLIFT CONNECTORS SHALL BE H10A U.N.O. ALL OVER FRAMED TRUSSES SHALL HAVE SIMPSON VTC 6" FROM EACH END, AND 48"O.C. TO TRUSSES BELOW

2 UNIT A1 2ND FLOOR FRAMING PLAN

1/4" = 1'-0"

2ND FLOOR FRAMING PLAN NOTES:

1. TRUSS BEARING ELEVATION IS + 10'-10 3/4"" U.N.O.
2. H-# INDICATES WOOD HEADER, SEE SCHEDULE ON SHEET S0.1
3. SEE S0.1 FOR WALL FRAMING SCHEDULE.
4. RUNS FOR MECH'L, ELECTRICAL AND PLUMBING (MEP) THROUGH PRE-FABRICATED TRUSSES MUST BE COORDINATED WITH THE TRUSS DESIGNER AND MEP DWGS. (PIPING, DUCT RUNS, ETC.) FIRE SPRINKLER RUNS MUST ALSO BE COORDINATED.
5. —(A)— DENOTES 18" FLOOR FRAMING AT 24" O.C. (TRUSS SPAN DIRECTION)
6. L# INDICATES MASONRY LINTEL, SEE SCHEDULE

1 UNIT A1 FOUNDATION PLAN

1/4" = 1'-0"

FOUNDATION PLAN NOTES:

1. ELEV. ±0'-0" IS REFERENCE ONLY. SEE CIVIL FOR TRUE NAVD ELEVATION.
2. VERIFY SLOPES AND STEPS WITH ARCH'L PRIOR TO CONSTRUCTION. SEE TYPICAL STEP DETAIL ON S4.1
2. G.C. TO PROVIDE SLAB ON GRADE CONTROL JOINTS (C.J.) FOR ALL SLAB AT 12'-0" O.C. MAX. TYP. FOR WALL AND SLAB CONTROL JOINTS. SEE S4.1 FOR DETAILS AND MORE INFO.
3. T.E. INDICATES THICKENED EDGE SEE S3.1 FOR DETAILS.
4. TS#, WF# INDICATE MONOLITHIC CONCRETE FOOTINGS, SEE SCHEDULE ON S0.1

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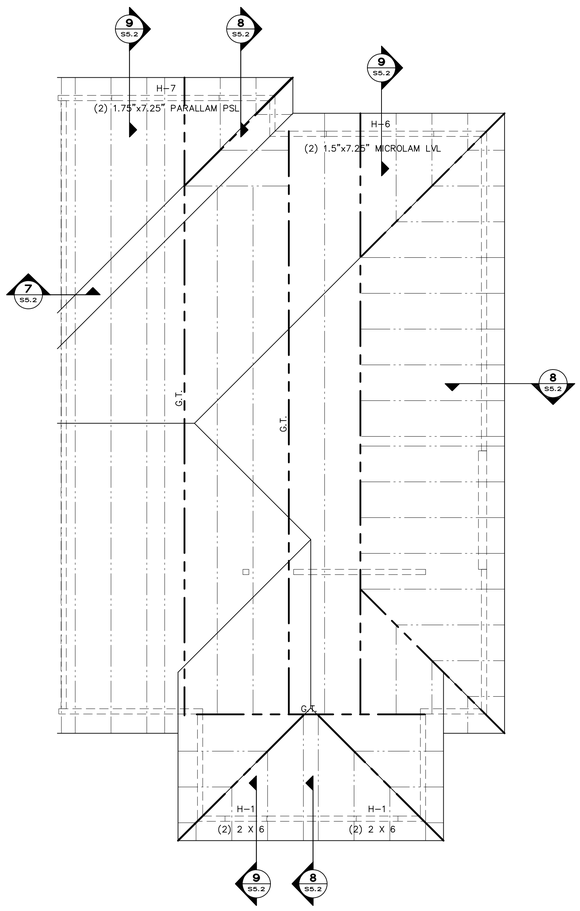
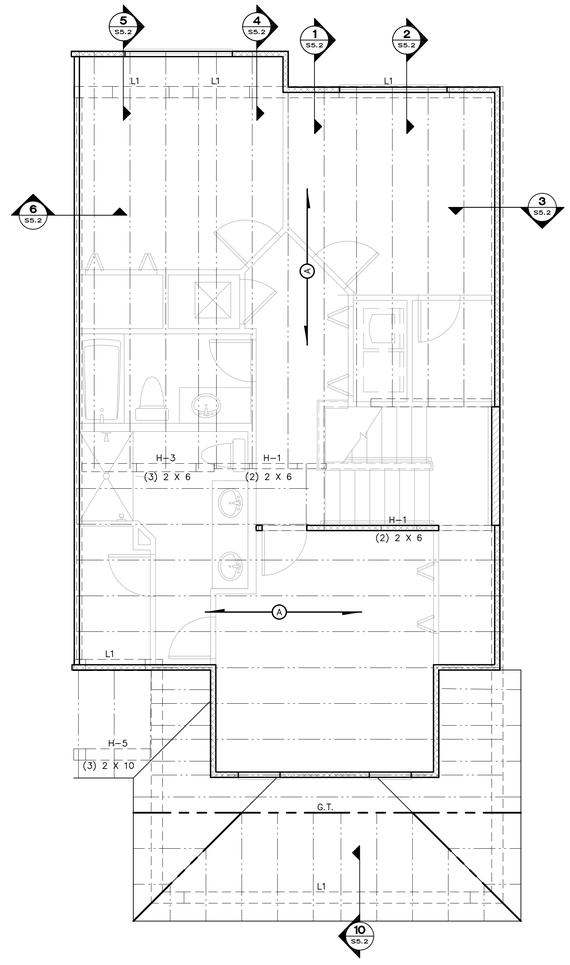
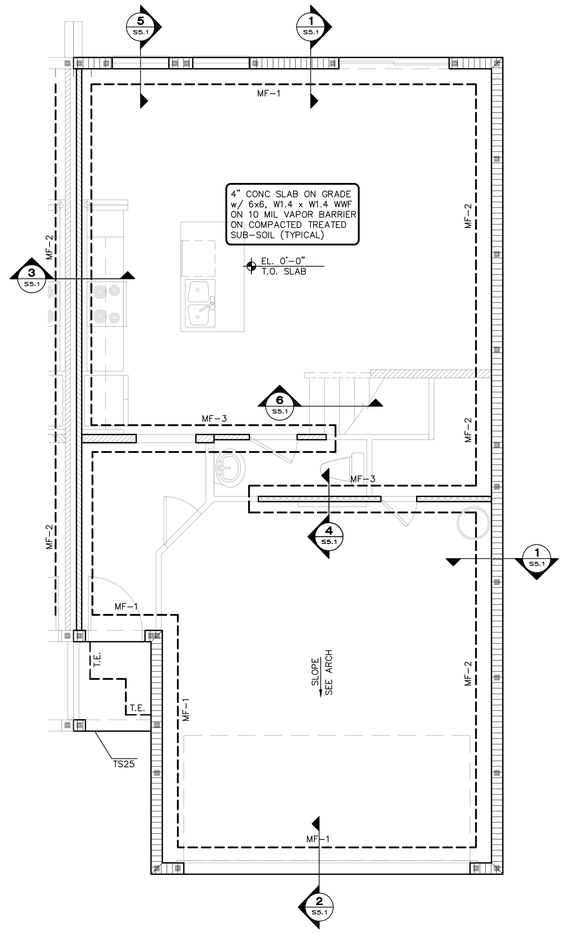
S1.1







3 UNIT A2 ROOF FRAMING PLAN

1/4" = 1'-0"

ROOF FRAMING NOTES:

1. FLOOR MEMBER BEARING ELEVATION IS +18'-11 7/8" U.N.O.
2. ROOF WOOD TRUSSES SHALL BE SPACED 2'-0" O.C. MAX.
3. G.T. INDICATES ROOF GIRDER TRUSS BY TRUSS MANUFACTURER.
4. H-# INDICATES WOOD HEADER, SEE SCHEDULE ON SHEET S0.1.
5. RUNS FOR MECH'L, ELECTRICAL AND PLUMBING (MEP) THROUGH PRE-FABRICATED TRUSSES MUST BE COORDINATED WITH THE TRUSS DESIGNER AND MEP DWGS. (PIPING, DUCT RUNS, ETC.) FIRE SPRINKLER RUNS MUST ALSO BE COORDINATED.
6. ALL ROOF TRUSS UPLIFT CONNECTORS SHALL BE H10A U.N.O. ALL OVER FRAMED TRUSSES SHALL HAVE SIMPSON VTC 6" FROM EACH END, AND 48"O.C. TO TRUSSES BELOW

2 UNIT A2 2ND FLOOR FRAMING PLAN

1/4" = 1'-0"

2ND FLOOR FRAMING PLAN NOTES:

1. TRUSS BEARING ELEVATION IS + 10'-10 3/4"" U.N.O.
2. H-# INDICATES WOOD HEADER, SEE SCHEDULE ON SHEET S0.1
3. SEE S0.1 FOR WALL FRAMING SCHEDULE.
4. RUNS FOR MECH'L, ELECTRICAL AND PLUMBING (MEP) THROUGH PRE-FABRICATED TRUSSES MUST BE COORDINATED WITH THE TRUSS DESIGNER AND MEP DWGS. (PIPING, DUCT RUNS, ETC.) FIRE SPRINKLER RUNS MUST ALSO BE COORDINATED.
5. —(A)— DENOTES 18" FLOOR FRAMING AT 24" O.C. (TRUSS SPAN DIRECTION)
6. L# INDICATES MASONRY LINTEL, SEE SCHEDULE

1 UNIT A2 FOUNDATION PLAN

1/4" = 1'-0"

FOUNDATION PLAN NOTES:

1. ELEV. ±0'-0" IS REFERENCE ONLY. SEE CIVIL FOR TRUE NAVD ELEVATION.
2. VERIFY SLOPES AND STEPS WITH ARCH'L PRIOR TO CONSTRUCTION. SEE TYPICAL STEP DETAIL ON S4.1
2. G.C. TO PROVIDE SLAB ON GRADE CONTROL JOINTS (C.J.) FOR ALL SLAB AT 12'-0" O.C. MAX. TYP. FOR WALL AND SLAB CONTROL JOINTS. SEE S4.1 FOR DETAILS AND MORE INFO.
3. T.E. INDICATES THICKENED EDGE SEE S3.1 FOR DETAILS.
4. TS#, WF# INDICATE MONOLITHIC CONCRETE FOOTINGS, SEE SCHEDULE ON S0.1

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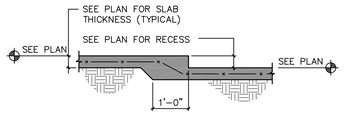
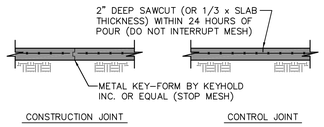
S2.1



TYPICAL SLAB RECESS

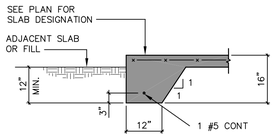
N.T.S.



TYPICAL SLAB-ON-GRADE

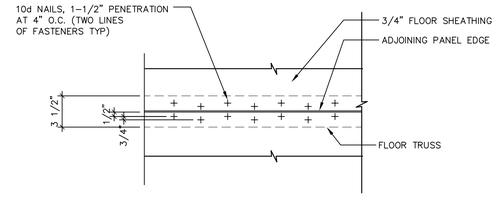
NOTE: CONTROL JOINTS/CONSTRUCTION JOINTS SHALL CREATE PANELS OF 400 SQ. FEET (MAXIMUM)

N.T.S.

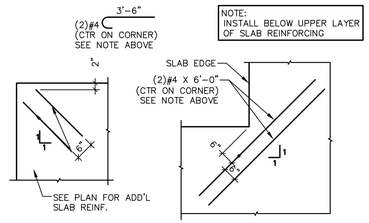


THICKENED EDGE (T.E.)

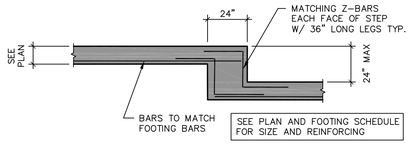
N.T.S.



TYPICAL FLOOR SHEATING ATTACHMENT DETAIL

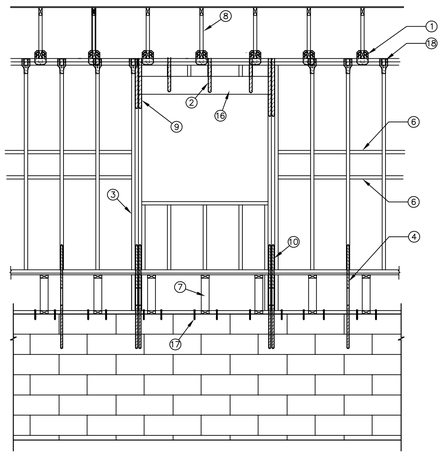


TYP. SLAB CORNER REINF.



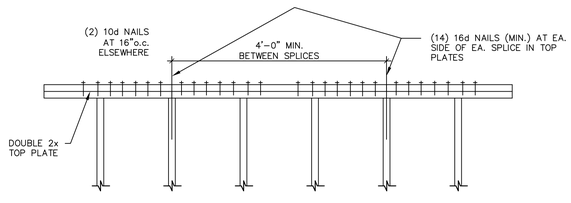
TYPICAL STEP FOOTING DETAIL

N.T.S.

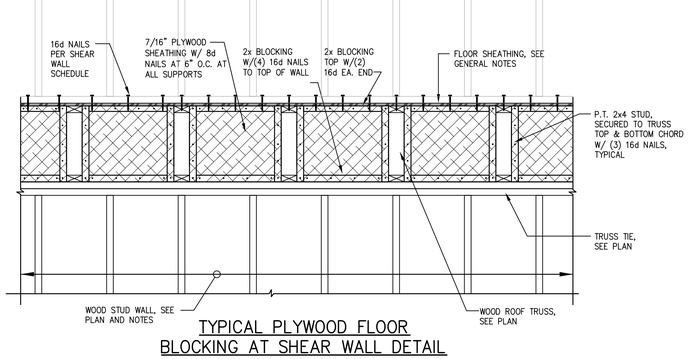


EXTERIOR WALL (TYP.)

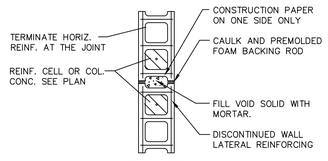
1. SIMPSON H10A PER PLAN
2. SIMPSON CS16 WRAPPED AROUND TOP PLATE, WITH 9 INCH END LENGTHS TO HEADER OR CRIPPLE STUDS BELOW HEADER
3. JAMB AND KING STUD FRAMING
4. SIMPSON MSTSM24 AT 32" O.C.
5. SHEAR WALL END ANCHOR TO TOP OF CMU WALL (ATTACHED TO SQUASH BLOCKS
6. BLOCKING FOR WALL SHEATHING EDGE NAILING
7. FLOOR TRUSS FRAMING
8. ROOF TRUSS FRAMING
9. (2) SIMPSON CS16 WITH 11" END LENGTHS WRAPPED AROUND TOP PLATE TO FASTEN EACH SIDE OF CRIPPLE AND KING STUD
10. (2) SIMPSON SP2 AT EACH STUD TO MASONRY WALL
12. SHEAR WALL COMPRESSION STUDS PER SHEAR WALL SCHEDULE (SET EACH SIDE OF EACH ROD TYP.)
13. SHEAR WALL COMPRESSION STUD SQUASH BLOCKS
16. OPENING HEADER
17. TITEN HD 1/2"ø TAPCON, (2) PER BLOCK
18. SIMPSON SP2 AT EACH STUD



TYPICAL TOP PLATE DETAIL



TYPICAL PLYWOOD FLOOR BLOCKING AT SHEAR WALL DETAIL



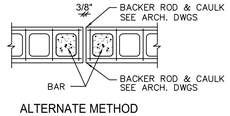
PLAN VIEW

NOTES :

1.- SAW CUT BOND BEAMS, TIE BEAMS 1" DEEP TO CONTINUE WALL CONTROL JOINT TO TOP OF WALL.

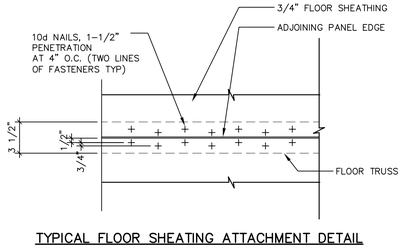
2.- CONTROL JOINT SPACING IS NOT TO EXCEED 25'-0"o.c. IN WALLS WITH MORE THAN 25'-0" OF UNINTERRUPTED MASONRY. REFER TO DWG'S. FOR ADDITIONAL SPECIFIED LOCATIONS AS NOTED THUS (WCJ).

3.- CONTINUE ALL BOND BEAMS, TIE BEAMS REINF. THROUGH THE JOINT.

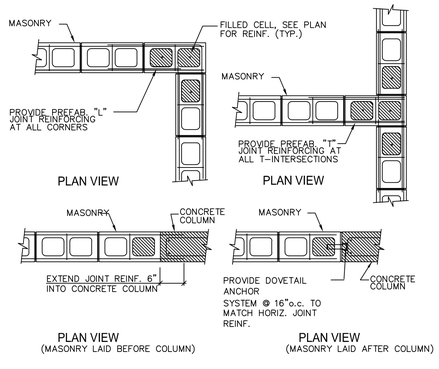


ALTERNATE METHOD

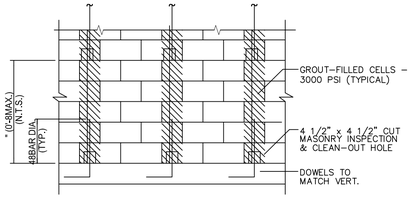
CMU WALL CONTROL JOINT (WCJ) DETAIL



TYPICAL FLOOR SHEATING ATTACHMENT DETAIL

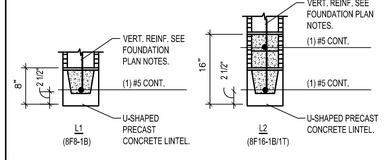


TYPICAL MASONRY DETAILS



TYPICAL MASONRY FILLED CELL DETAIL

8" MASONRY PRECAST LINTEL SCHEDULE



NOTES:

1. 8" PRECAST LINTELS BY CASTCRETE CORPORATION OR APPROVED EQUAL.
2. SHORE PRECAST LINTEL PER MANUFACTURER RECOMMENDATIONS.
3. SEE THE ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF ALL OPENINGS.
4. PROVIDE 4" MINIMUM BEARING EACH END.

MASONRY WALL SCHEDULE

MARK	THICKNESS	REINFORCING
MW1	8" CMU	#6 @ 32" O.C.
MW2	8" CMU	#6 @ 16" O.C.
MW3	8" CMU	#6 @ 8" O.C.
MW4	12" CMU	#7 @ 24" O.C.
MW5	12" CMU	#7 @ 16" O.C.
MW6	12" CMU	#7 @ 8" O.C.

MASONRY WALL NOTES:

1. WALL SEGMENTS SHALL BE REINFORCED WITH 9 GA. GALVANIZED LATERAL REINFORCING @ 16" O.C. HORIZ. EXTEND REINFORCING 8" INTO POURED ELEMENTS AND AROUND ENCASED STEEL.
2. ADJACENT TO ANY EXTERIOR/INTERIOR 8" WALL OPENING, PLACE 1 #6 VERTICAL IN CELL GROUTED SOLID, FULL HEIGHT. U.N.O. ON PLAN.
3. ADJACENT TO ANY EXTERIOR / INTERIOR 12" WALL OPENING, PLACE (4)#7 VERTICAL (TWO BARS, 6" APART IN EACH CELL) GROUTED SOLID, FULL HEIGHT, U.N.O. ON PLAN.
4. ALL MASONRY REINFORCED CELLS SHALL BE FILLED WITH 3000 PSI GROUT MIX.

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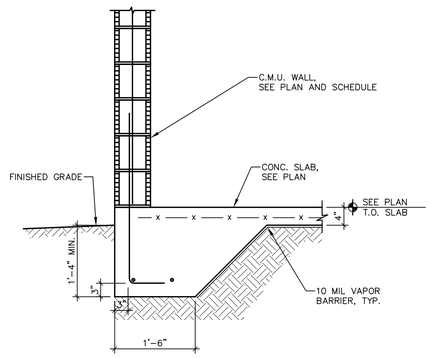
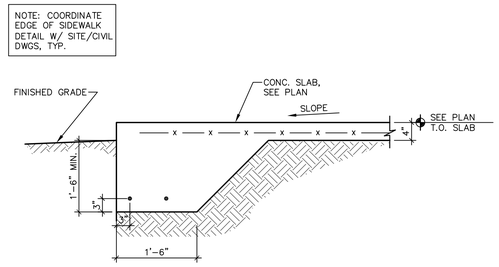
S4.1



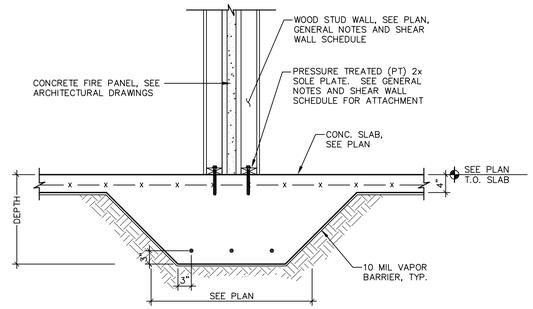
SECTION 1 — 3/4" = 1'-0" — S5.1

TYP. EXTERIOR WALL FOOTING



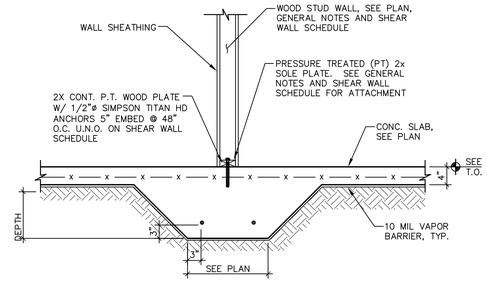
SECTION 2 — 3/4" = 1'-0" — S5.1

TYP. EXTERIOR BALCONY EDGE FOOTING



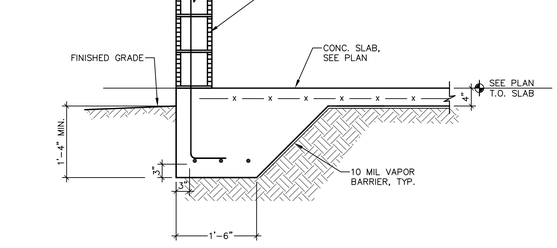
SECTION 3 — 3/4" = 1'-0" — S5.1

TYP. INTERIOR DBL WALL FOOTING



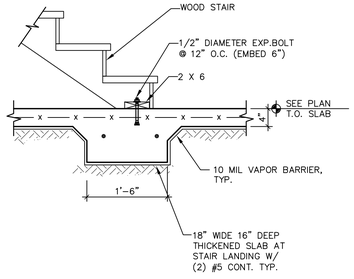
SECTION 4 — 3/4" = 1'-0" — S5.1

TYP. INTERIOR WALL FOOTING



SECTION 5 — 3/4" = 1'-0" — S5.1

TYP. EXTERIOR WALL FOOTING AT OPENING



SECTION 6 — 3/4" = 1'-0" — S5.1

TYPICAL WOOD STAIR FOOTING DETAIL
AT CONVENTIONAL CONCRETE SLAB

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LEVEL ELEVEN STUDIO INC.
220 SANDLEWOOD TRL
WINTER PARK, FL. 32789
407.519.9157

Narcoossee Reserve - SDP 20-0025
Townhomes
Thompkins Dr, Osceola County, FL 34771

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Matt Phelps
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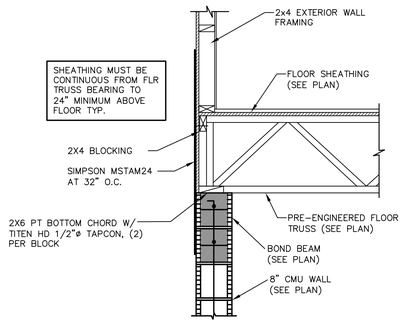
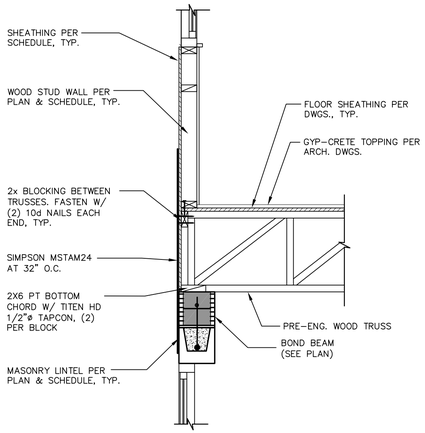
S5.1



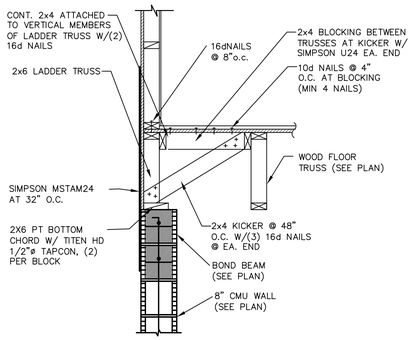
EXTERIOR UNIT BEARING WALL

SECTION 1 — 3/4" = 1'-0" — S5.2



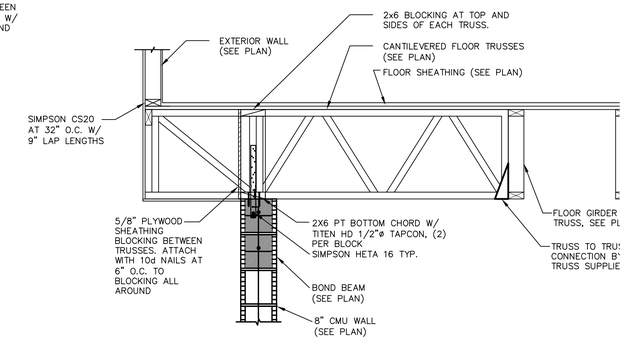
EXTERIOR UNIT BEARING HEADER

SECTION 2 — 3/4" = 1'-0" — S5.2

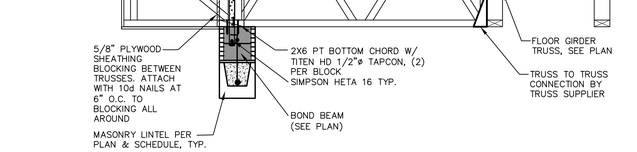


EXTERIOR UNIT NON-BEARING WALL

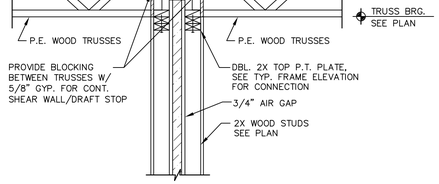
SECTION 3 — 3/4" = 1'-0" — S5.2



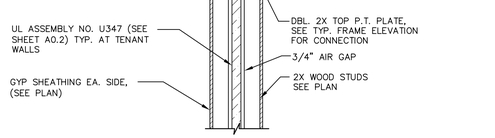
SECTION 4 — 3/4" = 1'-0" — S5.2



SECTION 5 — 3/4" = 1'-0" — S5.2

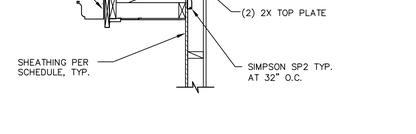


SECTION 6 — 3/4" = 1'-0" — S5.2



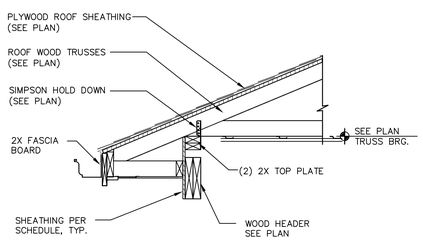
INTERIOR UNIT PARALLEL WALL

SECTION 7 — 3/4" = 1'-0" — S5.2



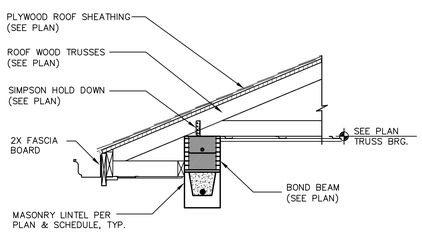
EXTERIOR UNIT BEARING WALL

SECTION 8 — 3/4" = 1'-0" — S5.2



EXTERIOR UNIT BEARING WALL

SECTION 9 — 3/4" = 1'-0" — S5.2



SECTION 10 — 3/4" = 1'-0" — S5.2

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S5.2